

The Principle of Minimal Potential Energy of Mixed Variables to Solve the Bending of Cantilever Rectangular Plate under Uniform Load

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Abstract. The problem of the balance of the cantilever rectangular thin plate under the uniform load is solved by using the minimum potential energy of mixed variables. The solution process is clear, and the deflection surface equation is given. Through the numerical calculation, the calculation results of the graph form are obtained, which are compared with the finite element results, and the accuracy of the numerical results is verified. It shows that the method presented in this paper has certain practical significance to the practical application of engineering, which can be directly applied to the actual project.

Introduction

With the development of the national economy, the construction industry is developing rapidly. Many construction projects involve the balance problems about a variety of elastic sheet and plates. The principle of energy gradually receives people's attention and is more and more popular in practical engineering applications because of its universality, simplicity and accuracy in solving various problems in practical engineering problems. However, for the present, some of the existing research methods can not meet the design requirements. In this paper, the balance of the cantilever rectangular thin plate is further studied by the minimum potential energy principle of mixed variables given in [1]. The numerical solution of the bending problem of the rectangular thin plate with hanging corners, one fixed edge and three free edges under the uniform load is given, and compared with the finite calculation results, and the results can be directly applied to engineering design.

The Minimum Potential Energy Principle of Mixed Variables for Curved Rectangular Thin Plate

Consider the bending of a free rectangular plate with two adjacent edges fixed under the distributed load, as shown in Figure 1(a). Release the fixed bending restraints at both ends, substitute the distribution bending moments \bar{M}_{x0} and \bar{M}_{y0} , respectively. The free edge deflection curve equations are denoted as \bar{w}_{xa} and \bar{w}_{yb} , respectively, and the suspension corner deflection is \bar{w}_{ab} , as shown in Figure 1(b).

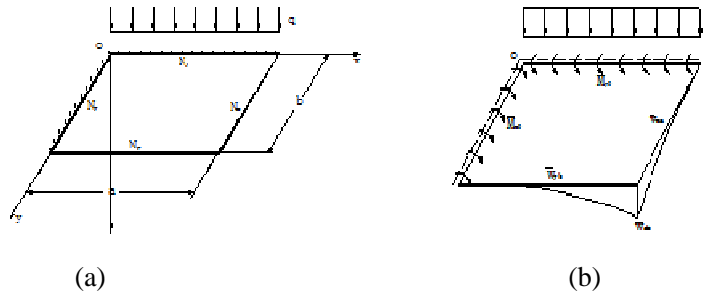


Fig. 1 The bending rectangular plate with two adjacent clamped edges and the other two free edges under uniform load

The minimum potential energy principle of the mixed variable is applied to the total potential energy of the rectangular slab in the graph:

$$\Pi_{mp} = \int_0^a \int_0^b \frac{1}{2} D \left[\left(\frac{\partial^2 w}{\partial x^2} + \frac{\partial^2 w}{\partial y^2} \right)^2 - 2(1-\nu) \left[\frac{\partial^2 w}{\partial x^2} \frac{\partial^2 w}{\partial y^2} - \left(\frac{\partial^2 w}{\partial x \partial y} \right)^2 \right] \right] dx dy -$$

$$\int_0^a \int_0^b q w dx dy - \int_0^b \bar{M}_{x0} \left(\frac{\partial w}{\partial x} \right)_{x=0} dy - \int_0^a \bar{M}_{y0} \left(\frac{\partial w}{\partial y} \right)_{y=0} dx + \int_0^b \bar{w}_{xa} (V_x)_{x=a} dy + \int_0^a \bar{w}_{yb} (V_y)_{y=b} dx - \bar{w}_{ab} R_{ab} \quad (1)$$

The upper part is called the total potential energy of the minimum potential energy of the mixed variable, or the combined total potential energy.

In the case of (1), take the variational extremes of w , V_{xa} , V_{yb} and R_{ab} , and apply the zero-variational principle of the inner boundary and the basic preparatory theorem of the variational method, the Euler equation and the natural boundary condition are obtained respectively:

$$D \left(\frac{\partial^4 w}{\partial x^4} + 2 \frac{\partial^4 w}{\partial x^2 \partial y^2} + \frac{\partial^4 w}{\partial y^4} \right) = q \quad (2) \quad -D \left(\frac{\partial^2 w}{\partial x^2} + \nu \frac{\partial^2 w}{\partial y^2} \right)_{x=0} = \bar{M}_{x0} \quad (3)$$

$$-D \left(\frac{\partial^2 w}{\partial x^2} + \nu \frac{\partial^2 w}{\partial y^2} \right)_{x=a} = 0 \quad (4) \quad -D \left(\frac{\partial^2 w}{\partial y^2} + \nu \frac{\partial^2 w}{\partial x^2} \right)_{y=0} = \bar{M}_{y0} \quad (5)$$

$$-D \left(\frac{\partial^2 w}{\partial y^2} + \nu \frac{\partial^2 w}{\partial x^2} \right)_{y=b} = 0 \quad (6) \quad (w_x)_{x=0} = 0 \quad (7)$$

$$(w_x)_{x=a} - \bar{w}_{xa} = 0 \quad (8) \quad (w_y)_{y=0} = 0 \quad (9)$$

$$(w_y)_{y=b} - \bar{w}_{yb} = 0 \quad (10) \quad (w_{xy})_{x=a, y=b} - \bar{w}_{ab} = 0 \quad (11)$$

$$(w_{xy})_{x=0, y=0} = (w_{xy})_{x=a, y=0} = (w_{xy})_{x=a, y=b} = 0 \quad (12)$$

The above is the minimum potential energy principle of the mixed variable of the rectangular plate.

Cantilever rectangular plate with two hanging corners, one fixed edge and three free edges

Considering that the rectangular plate is under uniform load, one edge is fixed and three edges are free, based on the previous text.

Assume that the deflection surface equation is.

$$w(x, y) = w_1(x, y) + w_2(x, y) + \frac{y}{b}k \quad (13)$$

among them, w_1 and w_2 is given in [1].

It is assumed firstly in the calculation of the rectangular plate with the one edge fixed and the other three edges free.

$$\bar{w}_{x0} = \bar{w}_{xa} = \sum_{n=1,2}^{\infty} a_n \sin b_n y + \frac{y}{b}k \quad (14) \quad \bar{w}_{yb} = \sum_{m=1,2}^{\infty} d_m \sin a_m x + k \quad (15)$$

$$\bar{M}_{y0} = \sum_{m=1,2}^{\infty} C_m \sin a_m x \quad (16)$$

And k is the deflection of the free point.

Apply w_1 and w_2 into the case of (1) and take the variational extremes of w , V_{x0} , V_{xa} , V_{y0} , V_{yb} and R_{ab} , simplify it, and according to the basic preparatory theorem of the variational method, the Euler equation and the natural boundary condition are obtained respectively :

$$D \left(\frac{\partial^4 w_1}{\partial x^4} + 2 \frac{\partial^4 w_1}{\partial x^2 \partial y^2} + \frac{\partial^4 w_1}{\partial y^4} \right) = \frac{q}{2} \quad (17) \quad D \left(\frac{\partial^4 w_2}{\partial x^4} + 2 \frac{\partial^4 w_2}{\partial x^2 \partial y^2} + \frac{\partial^4 w_2}{\partial y^4} \right) = \frac{q}{2} \quad (18)$$

$$-D \left(\frac{\partial^2 w_1}{\partial x^2} + \nu \frac{\partial^2 w_1}{\partial y^2} \right)_{x=0} = 0 \quad (19) \quad -D \left(\frac{\partial^2 w_1}{\partial x^2} + \nu \frac{\partial^2 w_1}{\partial y^2} \right)_{x=a} = 0 \quad (20)$$

$$-D \left(\frac{\partial^2 w_2}{\partial y^2} + \nu \frac{\partial^2 w_2}{\partial x^2} \right)_{y=0} = \bar{M}_{y0} \quad (21) \quad -D \left(\frac{\partial^2 w_2}{\partial y^2} + \nu \frac{\partial^2 w_2}{\partial x^2} \right)_{y=b} = 0 \quad (22)$$

$$w_{1x0} = \bar{w}_{x0} \quad (23) \quad w_{1xa} - \bar{w}_{xa} = 0 \quad (24)$$

$$w_{2y0} = 0 \quad (25) \quad w_{2yb} - \bar{w}_{yb} = 0 \quad (26)$$

substitute w_1 , w_2 into (17) ~ (26), calculate and simplify to get the deflection surface equation:

$$w(x, y) = \frac{4q}{Da} \sum_{m=1,2}^{\infty} \left\{ 1 + \frac{1}{2 \cosh \frac{1}{2} \alpha_m b} \left[\alpha_m \left(y - \frac{b}{2} \right) \operatorname{sh} \alpha_m \left(y - \frac{b}{2} \right) - \left(2 + \frac{1}{2} \alpha_m b \tanh \frac{1}{2} \alpha_m b \right) \operatorname{ch} \alpha_m \left(y - \frac{b}{2} \right) \right] \right\} \frac{[1 + (-1)^{m+1}]}{\alpha_m^5} \sin \alpha_m x +$$

$$\begin{aligned}
& \left[or \frac{4q}{Db} \sum_{n=1,2}^{\infty} \left\{ 1 + \frac{1}{2 \operatorname{ch} \frac{1}{2} b_n a} \left[b_n \left(x - \frac{a}{2} \right) \operatorname{sh} b_n \left(x - \frac{a}{2} \right) - \left(2 + \frac{1}{2} b_n a \operatorname{th} \frac{1}{2} b_n a \right) \operatorname{ch} b_n \left(x - \frac{a}{2} \right) \right] \right\} \frac{[1 + (-1)^{n+1}]}{b_n^5} \sin b_n y + \right. \\
& \left. \frac{1}{2D} \sum_{m=1,3}^{\infty} \left(-\frac{a_m b}{\operatorname{sh}^2 a_m b} \operatorname{sh} a_m y + \operatorname{cth} a_m b a_m y \operatorname{ch} a_m y - a_m y \operatorname{sh} a_m y \right) \frac{C_m}{a_m^2} \sin a_m x + \right. \\
& \left. \frac{1}{2} \sum_{n=1,2}^{\infty} \left\{ 2 \left(\operatorname{ch} b_n x - \operatorname{th} \frac{1}{2} b_n a \operatorname{sh} b_n x \right) + (1-m) \left[\operatorname{th} \frac{1}{2} b_n a \cdot \left(\frac{b_n a}{\operatorname{sh} b_n a} \operatorname{sh} b_n x + b_n x \operatorname{ch} b_n x \right) - b_n x \operatorname{sh} b_n x \right] \right\} a_n \sin b_n y + \right. \\
& \left. \frac{1}{2} \sum_{m=1,3}^{\infty} [2 + (1-m)(a_m b \operatorname{cth} a_m b - a_m y \operatorname{cth} a_m y)] \frac{d_m}{\operatorname{sh} a_m b} \operatorname{sh} a_m y \sin a_m x + \frac{y}{b} k \right. \quad (27)
\end{aligned}$$

The boundary conditions to be satisfied in this paper are:

$$\left(\frac{\partial w}{\partial y} \right)_{y=0} = 0 \quad (28) \quad -D \left[\frac{\partial^3 w}{\partial x^3} + (2-m) \frac{\partial^3 w}{\partial x \partial y^2} \right]_{x=0,a} = 0 \quad (29)$$

$$-D \left[\frac{\partial^3 w}{\partial y^3} + (2-m) \frac{\partial^3 w}{\partial x^2 \partial y} \right]_{y=b} = 0 \quad (30) \quad -2D(1-m) \left(\frac{\partial^2 w}{\partial x \partial y} \right)_{x=a,y=b} = 0 \quad (3)$$

1)

The deflection surface equation(27) is substituted into the above boundary conditions (28) - (31), and then the corresponding execution equations of the boundary conditions of the rectangular plate under the boundary condition are obtained as follows:

$$\begin{aligned}
& \frac{2q}{Db} \left(\operatorname{th} \frac{1}{2} a_m b - \frac{a_m b}{2 \operatorname{ch}^2 \frac{1}{2} a_m b} \right) \frac{1}{a_m^4} + \frac{1}{2D} \left(\operatorname{ch} a_m b - \frac{a_m b}{\operatorname{sh} a_m b} \right) \frac{C_m}{a_m \operatorname{sh} a_m b} + \frac{1}{2} [1 + a_m b \operatorname{cth} a_m b + m(1 - a_m b \operatorname{cth} a_m b)] \frac{a_m d_m}{\operatorname{sh} a_m b} + \\
& \frac{2}{a} \sum_{n=1,2}^{\infty} \frac{a_m}{K_{mn}^2} [a_m^2 b_n + (2-m)b_n^3] a_n + \frac{4}{ab} \frac{1}{a_m} k = 0 \quad (32)
\end{aligned}$$

$$\begin{aligned}
& \frac{q}{b} [1 + (-1)^{n+1}] \cdot \left[(3-m) \operatorname{th} \frac{1}{2} b_n a - (1-m) \frac{b_n a}{2 \operatorname{ch}^2 \frac{1}{2} b_n a} \right] \frac{1}{b_n^2} + \frac{1}{2} [(1 + b_n a \operatorname{cth} b_n a) + m(1 - b_n a \operatorname{cth} b_n a)] \frac{b_n}{\operatorname{sh} b_n a} A_n + \\
& \frac{2}{b} \sum_{m=1,3}^{\infty} \frac{a_m}{K_{mn}^2} [a_m^2 b_n + (2-m)b_n^3] C_m + \frac{D}{2} \left[-2(1-m^2) + (1-m)^2 \left(\frac{b_n a}{\operatorname{sh} b_n a} - 1 \right) \right] b_n^3 \operatorname{th} \frac{1}{2} b_n a a_n - \quad (33)
\end{aligned}$$

$$\frac{2q}{b} \left[-(3-m) \operatorname{th} \frac{1}{2} a_m b + (1-m) \frac{a_m b}{2 \operatorname{ch}^2 \frac{1}{2} a_m b} \right] \frac{1}{a_m^2} - \frac{1}{2} [(1 + a_m b \operatorname{cth} a_m b) + m(1 - a_m b \operatorname{cth} a_m b)] \frac{a_m}{\operatorname{sh} a_m b} C_m +$$

$$\frac{4D}{a}(1-m)^2 \sum_{n=1,2}^{\infty} \frac{(-1)^n a_m^3 b_n^3}{K_{mn}^2} a_n + \frac{D}{2} \left[2(1-m^2) \operatorname{cha}_m b + (1-m)^2 \left(\operatorname{cha}_m b + \frac{1}{\operatorname{sha}_m b} \right) \right] \frac{a_m^3}{\operatorname{sha}_m b} d_m = 0 \quad (34)$$

$$-4(1-m) \frac{q}{a} \sum_{m=1,2}^{\infty} \left(\operatorname{th} \frac{1}{2} a_m b - \frac{a_m b}{2 \operatorname{ch}^2 \frac{1}{2} a_m b} \right) \frac{1}{a_m^3} + (1-m) \sum_{m=1,3}^{\infty} (1-a_m b \operatorname{cha}_m b) \frac{1}{\operatorname{sha}_m b} C_m +$$

$$D(1-m) \sum_{n=1,2}^{\infty} \left[\left(\frac{b_n a}{\operatorname{sh} b_n a} - 1 \right) - n \left(\frac{b_n a}{\operatorname{sh} b_n a} + 1 \right) \right] \operatorname{th} \frac{1}{2} b_n a b_n^2 (-1)^n a_n + D(1-m) \sum_{m=1,3}^{\infty} \left[\left(\operatorname{cha}_m b + \frac{a_m b}{\operatorname{sha}_m b} \right) + n \left(\operatorname{cha}_m b - \frac{a_m b}{\operatorname{sha}_m b} \right) \right] \frac{a_m^2}{\operatorname{sha}_m b} d_m = 0 \quad (35)$$

Numerical Calculations

As a numerical example, take the square sheet of $a = b = 1, H = 1, m = 0.3$, and $a_n = b_n, d_m$. This example contains four unknown parameters C_m, a_n, d_m, k . And the iterative improvement method is used to solve it. Using Matlab software programming to calculate the available parameters A_n, C_m, b_n, d_m, k .

Table 1 Table of deflection values at $y = b \quad (qa^4/D) \times 10^{-1}$

x/a		0.05	0.15	0.25	0.35	0.45	0.50	0.70	0.90	1.00
w_{yb}	Ansysis 值	1.290	1.291	1.294	1.296	1.297	1.297	1.296	1.291	1.289
	本文	1.290	1.292	1.295	1.296	1.297	1.297	1.296	1.291	1.289

Table 2 Table of deflection values at $x = 0 \quad (qa^4/D) \times 10^{-1}$

y/b		0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	1.0
w_{x0}	Ansysis 值	0.002	0.082	0.180	0.302	0.447	0.604	0.770	0.941	1.289
	本文	0.002	0.083	0.180	0.303	0.447	0.604	0.770	0.941	1.289

Table 3 Computational results of bending moment at $y = 0 \quad (-qa^2)$

x/a	0.05	0.10	0.15	0.20	0.35	0.50	0.70	0.90
M_{y0}	0.4569	0.5246	0.5326	0.5326	0.5326	0.5325	0.5326	0.5246

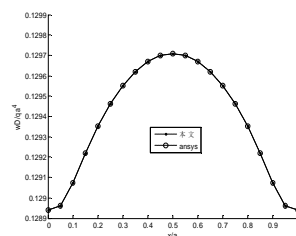


Fig. 2 Deflection curves at $y = b$

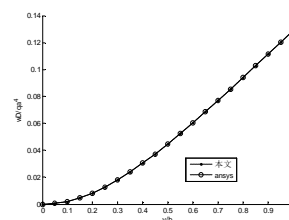


Fig.3 Deflection curves at $x = 0$

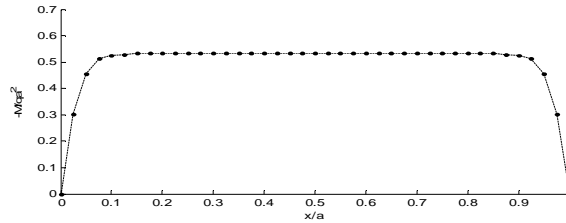


Fig. 4 Graph of bending moment values at $y = 0$

The numerical solution of the deflection function is obtained by substituting the results in the deflection surface equation, and the results are shown in Table 1 and Table 2. Table 1 and Table 2 gives the numerical solution of the deflection changing along x and y for the free edge $y=b$ and $x=0$, Figure 2 and Figure 3 is for the distribution curve of the deflection for the free edge $y=b$ and $x=0$.

Substitute C_m into (14) and (16), and we can get the distribution value along x about the bending moment of the fixed edge $y = 0$, and the results are shown in Table 3. Figure 4 is for the distribution curve of the bending moment for the fixed edge $y = 0$.

The finite element software ANSYS is used to analyze the cantilever rectangular plate. Taking the example: length is $a=b=1$, thickness is $t=0.005$, elastic modulus $E = 210\text{GPa}$, uniform load $q = 1\text{N/m}^2$. The 4-node shell unit is chosen to convert the computer results into a dimensionless quantity. Considering the bending stiffness of the sheet is $D = Et^3 / [12(1-\nu^2)] = 1230000\text{N/m}$, and this result is consistent with the result of the minimum potential energy principle of the mixed variable. The results are shown in Table 1 and Table 2 and Table 3, which show that the application of the minimum potential energy principle of mixed variables to solve the problem of rectangular thin plate bending is correct.

Conclusion

In this paper, the minimum potential energy principle of the mixed variable is used to solve the problem of the bending of the cantilever rectangular thin plate with two hanging corners, one fixed edge and three free edges under the uniform load. The deflection surface equation and the calculation result are given and compared with the finite element results. It is proved that the solution of the minimum potential energy principle of mixed variable is very effective and the result given is accurate. Therefore, the minimum potential energy principle of the mixed variable provides a new method for the solution of the elastic mechanics problem, which can be applied directly to the engineering practice.

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