

# Damage index of the longitudinal connection of simple supported-to-continuous girder bridges constructed with prestressed hollow slabs

Jing Liu<sup>1, 2 a</sup>, Jinquan Zhang<sup>2, b</sup>, Wanheng Li<sup>2, c</sup> and Bo Diao<sup>1, d</sup>

<sup>1</sup>School of Transportation Science and Engineering, Beihang University, Beijing 100191, China

<sup>2</sup>Research Institute of Highway, MOT, Beijing 100088, China

<sup>a</sup>liu.jing@rioh.cn, <sup>b</sup>jq.zhang@rioh.cn, <sup>c</sup>wh.li@rioh.cn, <sup>d</sup>diaobo@buaa.edu.cn

**Keywords:** Damage index, simple supported-to-continuous girder, hollow slab, longitudinal connection.

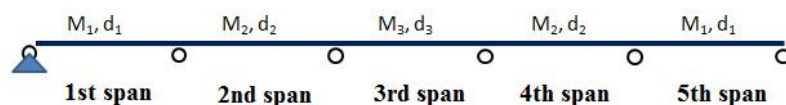
**Abstract.** Simple supported-to-continuous girder bridges are commonly and widely used in bridge engineering in China. For this type of bridges, normally, the longitude connections are casted in-site by concrete. The quality of the casted concrete is difficult to control. After a large number of field surveys, damages are often found in the transversal and longitudinal connections. These damages may result in unfavorable effects to the bridge system and shorten the durability of the bridges. Therefore it is of great interests to investigate the behavior of the concoctions on this kind of bridges. This paper presents a numerical study of the longitudinal connections of simple supported-to-continuous girder bridges. Then by series parametric study, the effect of the damages on the connections on the mechanical behavior of the bridge is investigated. And the damage indexes  $d$  and  $M$  are proposed in the evaluation of the damage of continuum bridges.

## Introduction

For the reasons of easily construction and economic, simple supported-to-continuous girder bridges are commonly and widely used in bridge engineering in China. For this type of bridges, normally, the longitude connections are casted in-site by concrete. The quality of the casted concrete is difficult to control [1-4]. After a large number of field surveys, damages are often found in the transversal and longitudinal connections. These damages may result in unfavorable effects to the bridge system and shorten the durability of the bridges [5]. Therefore it is of great interests to investigate the behavior of the concoctions on this kind of bridges. Now, only a few qualitative methods for the assessment of the longitudinal connection are available in JTG/T H21-2011 and JTG/T J21-2011 [6,7]. This study presents a numerical study on the effect of strength reduction of the connections on the performance of the bridges. Based on the results, a damage assessment method is proposed for the longitudinal connection of simple supported-to-continuous girder bridges.

## Numerical modeling parameters

In this section, the numerical modeling parameters will be presented first. The numerical calculation model is based on the Chinese standard manual for prestressed hollow section slab bridge, in which a prototype of a 16 m with 5 spans is selected as the example, see Fig. 1.  $M_1 \sim M_3$ 、 $d_1 \sim d_3$  are bending moment and deflection in the middle of the spans, respectively.



**Figure 1. Calculation model of a simple supported-to-continuous girder bridge**

The load applied on the calculation model is based on the Chinese standard (JTG D60-2004). The internal forces are calculation as a class structural member. The load applied on slab is re-calculated using a load transfer factor. The connection of the slab and concrete girder is regarded as stiff moment

connection. The numerical calculation is carried out by FEM method using truss or beam element. The damage of the longitude connection is considered by strength reduction of the concrete.

### Damage index $\xi_M$ of longitudinal connection based on variation of internal force

The calculation of damage index  $\xi_M$  is based on the FEM results obtained in the previous section. In this study the damage of the concrete is defined as the reduction of the stiffness of the concrete. For the convenience of analysis, the factors of damage  $D_{EI}$  and  $\mu_M$  are defined as  $D_{EI}=1-(EI)/(E_0I_0)$ ;  $\mu_M=M/(M_0)$ . ( $E_0I_0$ ) and  $M_0$  are the initial stiffness and bending moment when there is no damages. The damage index is  $\xi_M$  defined as the bending moment of span 1 and span 2.  $\xi_M = M_2/M_1$ . With the increasing of the bending moment  $M_1$  on span 1, with the damage increase, the bending moment on the nearby span  $M_2$  was calculated, see in Table 1

TABLE 1.  $\xi_M$  CALCULATED FROM FEM MODEL WITH INCREASING  $M_1$

$D_{EI}$	$M_1$ (kN.m)	$M_2$ (kN.m)	$\xi_M$
0.0	391.310	-106.882	-0.27
0.1	395.908	-96.878	-0.24
0.2	397.177	-89.187	-0.22
0.3	398.747	-81.268	-0.20
0.4	400.745	-73.024	-0.18
0.5	403.370	-64.289	-0.16
0.6	407.013	-54.763	-0.13
0.7	412.365	-43.792	-0.11
0.8	421.071	-29.844	-0.07
0.9	438.030	-7.781	-0.02

From the results, it can be seen that for a 5-span simple supported-to-continuous girder bridges, with the increasing of the applied bending moment, the damage factor  $\xi_M$  increases. For other cases, with the bending moment applied on the side span or middle span, the ratio of the bending moment of side span to the middle span is shown in Fig.2.

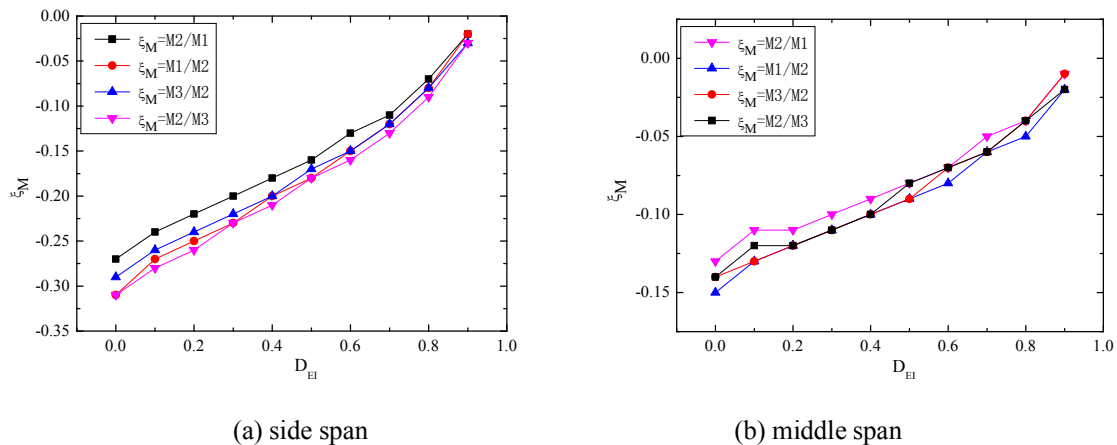


Figure 2. The relationship of  $\xi_M$  and  $D_{EI}$  of a 16m bridge

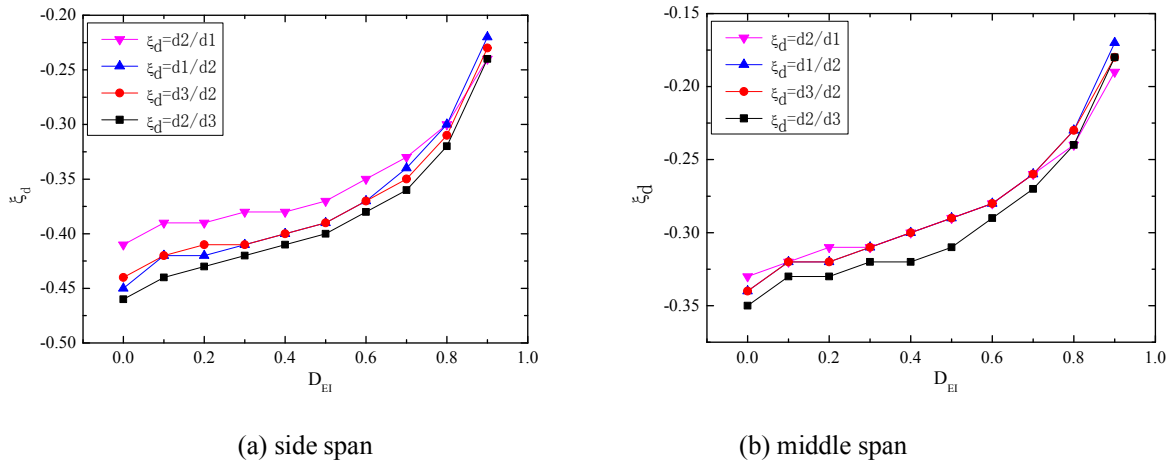
### Damage index $\xi_d$ of longitudinal connection based on variation of bridge deformation

The calculation of damage index  $\xi_M$  is based on the FEM results obtained in the previous section. In this study the damage of the concrete is defined as the reduction of the stiffness of the concrete. The damage index is  $\xi_M$  defined as the bending moment of span 1 and span 2.  $\xi_M = d_2/d_1$ . With the increasing of the deflection  $d_1$  on span 1, with the damage increase, the deflection on the nearby span  $d_2$  was calculated, see in Table 2. The factors of damage  $D_{EI}$  and  $\mu_M$  are defined as  $D_{EI}=1-(EI)/(E_0I_0)$ ;  $\mu_M=M/(M_0)$ . ( $E_0I_0$ ) and  $M_0$  are the initial stiffness and bending moment when there is no damages.

**TABLE 2.  $\xi_d$  CALCULATED FROM FEM MODEL WITH INCREASING  $d_1$**

$D_{EI}$	$d_1$ (mm)	$d_2$ (mm)	$\xi_d$
0.0	-3.490	1.430	-0.41
0.1	-3.536	1.396	-0.39
0.2	-3.550	1.384	-0.39
0.3	-3.568	1.372	-0.38
0.4	-3.590	1.354	-0.38
0.5	-3.620	1.330	-0.37
0.6	-3.660	1.296	-0.35
0.7	-3.722	1.243	-0.33
0.8	-3.824	1.151	-0.30
0.9	-4.025	0.949	-0.24

From the results, it can be seen that for a 5-span simple supported-to-continuous girder bridges, with the increasing of the applied bending moment, the damage factor  $\xi_d$  increases. For other cases, with the bending moment applied on the side span or middle span, the ratio of the bending moment of side span to the middle span is shown in Fig.3.



**Figure 3. The relationship of  $\xi_d$  and  $D_{EI}$  of a 16m bridge**

## Conclusions

This paper presents a numerical study of the longitudinal connections of simple supported-to-continuous girder bridges. Then by series parametric study, the effect of the damages on the connections on the mechanical behavior of the bridge is investigated. And the damage indexes  $\xi_d$  and  $\xi_M$  are proposed in the evaluation of the damage of continuum bridges. It can be concluded that with the damage developing, the damage indexes  $\xi_d$  and  $\xi_M$  will increase. The proposed damage indexes  $\xi_d$  and  $\xi_M$  are helpful for the practical application in bridge engineering in China.

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